



WATER DEMAND PROJECTIONS

EAST SIDE WATER TREATMENT SUPPLY PLANNING AND PROJECT DEVELOPMENT

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Madison, Wisconsin
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Madison, WI 53713

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B&V Project 169092.0100
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1. BACKGROUND

The Madison Water Utility (MWU) is developing a plan to continue to provide a reliable supply of high quality water cost effectively to the City of Madison's (City's) Zone 6 - East Service Area. The Zone 6 - East Service Area is served by six wells including Unit Well Nos. 7, 8, 9, 11, 13, 15, 23, 25 and 29.

As part of the East Side Water Supply Planning and Project Development Project (East Side Project), the purpose of this memorandum is to:

- Discuss conservation planning for the MWU and how it compares to other similar communities
- Document the assumptions used in developing and updating water demand projections and peaking factors for the MWU for Design Years 2010, 2015, 2030 and Buildout.
- Compare projected demands with existing available water supply.

2. STUDY AREA

The study area for this project is the City's East Service area including service zones 1, 3, 4, 5, and 6E. For the water demand projections, however, the boundary is the same as the 2006 Master Plan. The study areas referenced in this document are shown in Attachment A at the end of this memo and include the following:

- The City of Madison
- The Village of Maple Bluff (Maple Bluff)
- The Village of Shorewood Hills (Shorewood Hills)
- The Town of Madison
- Growth areas described in the City's 2005 Comprehensive Plan.
- East Service Area

3. CONSERVATION

Traditionally the north Midwestern United States has had abundant water supplies for municipal use and conservation has not been a priority. However, growing demand for water supply is beginning to put a strain on water resources and affect water quality. In addition, a growing understanding of the cost of wasteful water use including: capital improvements, water treatment, and declining water quality as a result of over pumping are prompting many communities to place an increased emphasis on conservation.

Although indoor water usage tends to be fairly stable throughout the year, outdoor water demand varies significantly seasonally and responds directly to weather patterns. Crediting only conservation without taking climatic trends into account for an overall reduction in water demand in the past 5 years may be pre-mature due to the wet cool summer weather pattern that has been prevalent in the Madison area. It is difficult to quantify the success of conservation efforts based on the total demand in the short-term since typically outdoor water demands vary considerably from year to year in response to temperatures and rainfall. Water demands will continue to be monitored over the next decade to evaluate overall conservation success.

3.1. MWU Conservation Efforts

Water conservation is not a new concept to MWU. Water conservation in Madison has a tradition reaching back more than 30 years to appropriate water use control techniques including but not limited to: metered water usage for all customers, leak detection and abatement programs, and an outdoor water use restriction ordinance (to control water use during emergency conditions). As a result, the City has relatively low per capita water use and water loss rates. In response to declining aquifer levels, impacts of well pumping on surface water features, and a desire to preserve the aquifer for generations to come, MWU adopted a Water Conservation and Sustainability Plan (Conservation Plan) in 2008. The Plan has a primary goal of maintaining the current annual rate of groundwater withdrawal in existing areas and secondary goals of:

- Residential: reduce residential water use by 20 percent by 2020 to an average use of 58 gallons per capita per day
- Commercial: promote water conservation through rebate promotions and education.
- Industrial: develop a water conservation plan for each industrial customer.
- Municipal: enact water savings programs for all government buildings that support the primary goal.

Interest in conservation has been in response to numerous factors including: reducing the need for adding additional or maintaining existing well capacity to the system, declining aquifer levels, impacting surface water features, contaminant transport, and the potential of declining water quality. In addition, there is a growing public awareness and demand for using natural resources in a sustainable manner. Water conservation not only saves water, it also reduces chemical usage and can provide a significant energy savings to a utility. Ultimately water conservation reduces MWU's overall carbon footprint. To be successful conservation efforts are implemented as a combination of public education, institutional regulations, monetary incentives and physical changes which results in a change in water use patterns within the general public.

In its Conservation Plan, MWU outlined the recommendations summarized in Table 1. In order to reduce residential usage by 20 percent, MWU will need to reduce the per capita usage from a 2002 – 2006 average of 73 gallons per capita per day (gpcd) to 58 gpcd (about 15 gpcd). Based on information from *Handbook of Water Use and Conservation: Homes, Landscapes, Industries, Businesses, Farms* (Amy Vickers, 2001) changing from standard toilets to high efficiency toilets will reduce water usage by approximately 10.3 gpcd, which is one of the easiest and most effective indoor water use conservation steps.

Table 1 - MWU Conservation Recommendations

Recommendation ¹	Description	Priority
Residential		
High efficiency toilets	MWU implemented a \$100 per household and apartment rebate program to replace old toilets with high efficiency "Water Sense" toilets	10/08 ²
Install an Advanced Metering Infrastructure (AMI) billing system	Install an AMI-system and start monthly billing.	Short Term ³
Provide customers with current	Instruct customers on tracking their water usage	Short Term ³

Table 1 - MWU Conservation Recommendations

Recommendation ¹	Description	Priority
consumption data through the AMI system	through meter reading.	
Inclining rate structure	Change the MWU rate structure to an inverted rate structure to reward low water usage and penalize high water usage	Short Term
Outdoor Water Usage Restrictions	Restrict outdoor water usage when pumping exceeds 50 mgd for 2 consecutive days.	Short Term
Residential water audit program	Allow individual residential customers to request an on-site or individual water audit of their home.	Long Term
High efficiency washing machines/dishwashers	Develop a financial incentive program for washing machines and dishwashers similar to the Utility's toilet rebate program	Long Term
Industrial		
Water Conservation Plans	Perform individual audits and develop water conservation plans for industrial customers	Short Term
Commercial		
Education	Target high-use customers with education/outreach to promote water conservation	Short Term
Landscaping ordinance	Enact landscaping ordinance with water limiting requirements and drought resistant plantings for new development/major redevelopment	Intermediate
Appliance Upgrade Program	Develop appliance upgrade program for heavy water use commercial clients	Long Term
Certification Program	Develop a certification program for water-efficient buildings	Long Term
Car Wash Reclamation Ordinance	Enact an ordinance requiring car washes to use water reclamation.	Long Term
Municipal		
Quantify Water Use	Improve record keeping to quantify water use for municipal accounts	Short Term
Minimize Reservoir Dumping	Improve operational control of water reservoirs to minimize dumping	Short Term
Leak-Detection Program	Expand leak detection program to identify and correct leaks	Short Term
Water Utility Bill	Upgrade water utility billing with new software	Short Term
Meter Raw Water Pumping	Install use meters in well buildings	Intermediate
Water Conservation Plans	Perform individual audits and develop water conservation plans for other government buildings	Intermediate
Reduce Hydrant Flushing	Reduce the Utility's annual unidirectional flushing program as well filters are installed, operational changes are implemented and overall water quality in the distribution system is improved	Short term to Intermediate

¹ Recommendations from the Conservation Plan, Summary of Conservation Goals Table.

² Recommendation has been implemented by MWU.

³ Madison will begin implementing a two-year AMI program in 2011 which will allow the Utility to move to monthly billing.

As MWU implements the Conservation Plan recommendations, the overall effectiveness of the program will be evaluated and the program will be refined and expanded as needed.

3.2. Conservation Efforts in Other Northern North American Communities

Although MWU has seen a reduction in water demands in the last couple of years, due to recent weather patterns, it is too early to evaluate the long-term effectiveness of the conservation program. For comparison, other northern mid-sized cities with established conservation programs and published results were selected and evaluated. Table 2 summarizes the conservation results from these communities.

The MWU Conservation Plan includes recommendations similar to other communities. Based on the City's historic demand rates and these examples, it will likely be difficult for Madison to achieve its 20 percent residential demand reduction goal by 2020 without a significant commitment of area residents to water conservation, an effective widespread education program, restrictions on outdoor water usage, the development of other conservation programs, and an expansion of the toilet retrofit rebate program.

**Table 2 - Comparison of Conservation Programs
for Northern North American Communities**

Utility	Start Year	Programs	Estimated Reduction in Water Demand
Lincoln, NE ¹	1988	<ul style="list-style-type: none"> • Increasing block rate structure • Public Education 	7 %
Waterloo, Ontario ²	Early 1980s	<ul style="list-style-type: none"> • Toilet retrofit • Water efficient shower heads 	13 %
Wichita, KS ³	1990s	<ul style="list-style-type: none"> • Toilet retrofit • 2 day per week watering • School education program • Proposed increasing block rate structure 	13% (projected)
Barrie, Ontario ⁴	1994	<ul style="list-style-type: none"> • Toilet retrofit • Water efficient shower heads 	7 % (16.5 gpcd)
Waukesha, WI ⁵	2006	<ul style="list-style-type: none"> • Toilet retrofit • Daytime irrigation ban • 2 day per week watering restriction • School education program • Proposed increasing block rate structure 	11%

¹ From www.lincoln.ne.gov/city/pworks/water/conserve/ and 2007 Facilities Master Plan Update (Black & Veatch, 2009).
² From *Regional Case Studies: Best Practices for Water Conservation in the Great Lakes-St. Lawrence Region* (Great Lakes Commission, June 2004)
³ From "IRP: A Case Study From Kansas," *Journal of the American Water Works Association* 87, No. 6 (June 1995): pp.57-71.
⁴ From *Cases in Water Conservation: How Efficiency Programs Help Water Utilities Save Water and Avoid Costs* (United States Environmental Protection Agency, 2002).
⁵ From "Waukesha, WI Promotes Water Conservation, Environmentally Responsible Water Supply Planning" by Mayor Larry Nelson, *U.S. Mayor Newspaper*, March 23, 2009 and "Proposed Waukesha Water Rates Encourage Conservation" by Lisa Kaiser, www.expressmilwaukee.com, Wednesday, May 20, 2009.

4. POPULATION AND EMPLOYMENT

Population and employment are important factors in evaluating existing water usage and projecting future water usage. Population and employment data by Traffic Analysis Zone (TAZ) are also used to develop an understanding of the spatial component (geography) of demands.

4.1. Historical, Existing, and Future Service Population

Estimates of existing service population were developed from Dane County TAZ projections and census estimates obtained from the Wisconsin Demographic Service Center and compared.

4.1.1 Methodology 1 –Demographic Service Center

The State of Wisconsin – Department of Administration Demographic Service Center (www.doa.state.wi.us) develops annual total population estimates for counties, towns, cities, and villages. Population estimates by year for the City, Maple Bluff, Shorewood, and the Town of Madison were obtained. In addition, to the incorporated areas served by MWU, there are approximately 8,000 customers located in unincorporated areas, called expansion areas. The Demographic Service Center does not provide employment information. Table 3 summarizes the historical population using this approach.

**Table 3 - Historical Population Estimates
from Demographic Service Center**

Year ¹	Town of Madison	City	Maple Bluff	Shorewood Hills	Expansion Area ²	Service Population
2000	6,611	207,248	1,339	1,659	8,000	224,857
2000 Census ³	7,005	208,054	1,358	1,732	8,000	226,149
2001	6,999	210,377	1,357	1,730	8,000	228,463
2002	6,974	213,679	1,357	1,729	8,000	231,739
2003	6,952	215,697	1,351	1,721	8,000	233,721
2004	6,936	217,935	1,350	1,724	8,000	235,945
2005	6,128	221,735	1,349	1,717	8,000	238,929
2006	6,104	223,280	1,342	1,711	8,000	240,437
2007	6,086	224,810	1,380	1,706	8,000	241,982
2008	6,033	226,650	1,378	1,699	8,000	243,760
2009	6,017	227,700	1,382	1,705	8,000	244,804
2010	5,923	228,200	1,384	1,701	8,000	245,208

¹ Estimated population as of January 1st of the indicated year.
² Expansion population was assumed and held constant based on a review of the 2006 Master Plan demographic data
³ 2000 United States Census Results.

4.1.2 Methodology 2 – Interpolation of TAZ Data

The Madison Area Transportation and Planning Board provided population and employment data by TAZ for years 2000, 2030, and 2035. The advantage of the TAZ data is that it provides not only a total service area population, but also provides information on the spatial distribution of the population and employment. TAZ data was combined with current city limit and anticipated service area boundaries to develop the population and employment projections for the service area. Linear interpolation was used between 2000 and 2035 to determine intermediate year values. This data is summarized on Table 4. Since the 2005 Comprehensive Plan developed by the Planning Department of the City of Madison is still in effect, there has been no change in the long-term land use projection. The buildout projections from the 2006 Water Master Plan have not changed and are still appropriate for use.

Using the TAZ data and East Side Area boundary, the population and employment growth for the East Side was also calculated and summarized in Table 4.

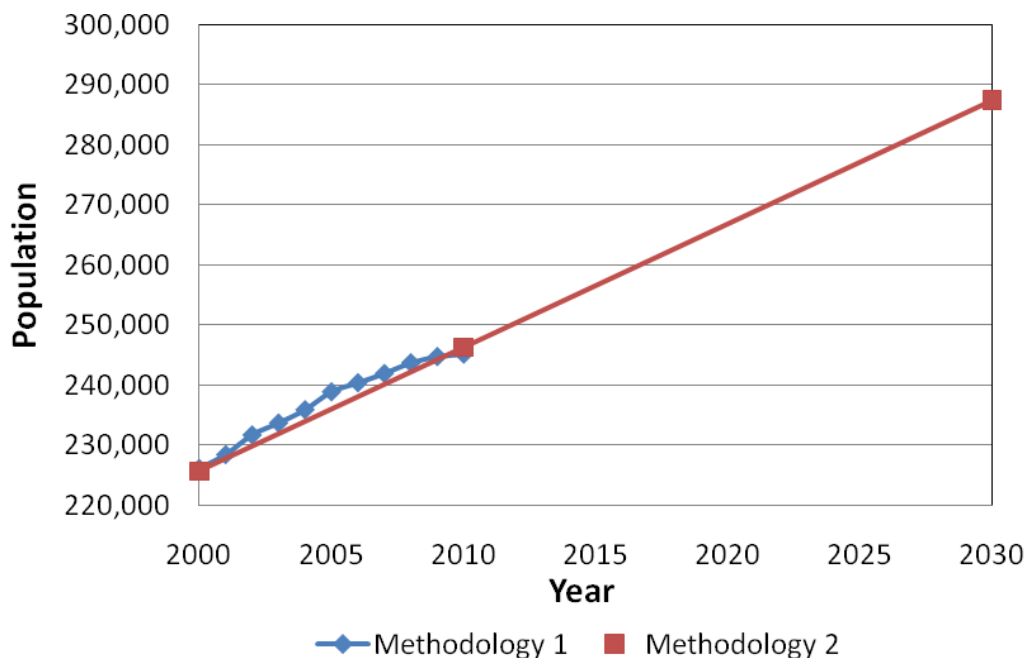
Table 4 - Projected Population and Employment

Year	System-Wide		East Side Area	
	Population ¹	Employment ¹	Population ²	Employment ²
2000	225,600	190,800	--	--
2010	246,300	214,400	85,300	66,100
2015	256,600	226,300	89,500	72,300
2030	287,500	261,700	102,100	90,900
Buildout ³	381,200	322,500	178,600	142,600

¹ Projection = $\{(2030 \text{ Projection} - 2000 \text{ Projection})/30 * (\text{year} - 2000)\} + 2000$ Projection
² Includes pressure zones 1, 3, 4, 5, and 6E.
³ Madison Water Utility Plan (Black & Veatch, December 2008), Table 2-5 and Table 2-15

The two approaches are shown graphically on Figure 1. Methodology 2 produces a population projections that is less than one percent higher (about 1,000 people) than projections from the Demographic Services Center. Because the TAZ projections are similar to the Demographic Services Center projections and also provide employment information and spatial distribution of population, Methodology 2 will be used for estimating existing population and employment.

Figure 1 - Population Methodology Comparison



Attachment B includes detailed information on population and employment by TAZ (and neighborhood for the East Side) along with a map identifying the TAZ locations.

5. WATER DEMANDS

Projected water demands are developed from existing water demands and the anticipated impact of growth and conservation on the demand.

5.1. Definitions and Usage

A water utility must be able to supply water at rates that fluctuate over a wide range. Yearly, seasonally, monthly, weekly, daily, and hourly variations in water demand occur in all water systems, with higher water use typically occurring during hot, dry weather due to increased outdoor use. Water use rates follow a daily (diurnal) pattern that will vary by season and day of the week. Water demand is typically lowest at night and greatest in the early morning and late afternoon. The importance of the key demand rates to the hydraulic design and operation of a water supply and distribution system are as follows:

- **Average Day (AD) Demand:** The AD demand rate is used primarily as the basis from which to estimate maximum day (MD) and maximum hour (MH) demands. The AD rate is also used to estimate future revenues and operating costs. The AD demand rate is calculated as the total volume of water used during the year, divided by the number of days in the year.
- **Summer Demand (SD):** This gives insight into the additional pumping required in the summer and the amount of water used in outdoor applications. It is calculated as the water volume used during the highest 3 months of pumping divided by 90.
- **Maximum 30 Day (M30D) Demand:** Also called maximum month, the average rate of use during the M30D is a good indicator of the period in which the MD use rate will be found. It also indicates the season of elevated use over a prolonged period, which is used to evaluate the ability of the source of supply to yield adequate quantities of water over extended periods. It is calculated as the maximum volume of water used in a single month divided by 30.
- **Maximum 10 Day (M10D) Demand:** The M10D is the average rate of use during the maximum 10 day period. It is calculated as the maximum value of water used in a 10 day period divided by 10. The M10D demand will be used in hydraulic modeling efforts in future tasks of this project. This demand level is typically indicative of what happens when the system is highly stressed and serves to demonstrate the water systems ability to meet MWUs level of service.
- **Maximum 7 Day (M7D) Demand:** The M7D is the average rate of use during a maximum 7 day period. It is calculated as the maximum value of water used in a 7 day period divided by 7.
- **Maximum Day:** The MD rate is used to size water supply and treatment facilities, and booster pumping stations when equalization storage is properly sized. The MD demand distribution is combined with fire flow demand at selected locations to assess the maximum hydraulic capacity of the distribution system to satisfactorily serve required fire demand. It is calculated as the maximum volume of water used during a single day of the year.
- **Maximum Hour:** Since minimum distribution system pressures are usually experienced during MH, the sizes and locations of distribution facilities are generally determined on the basis of this condition. MH water requirements are partially met through the use of strategically located system storage. The use of system storage minimizes the required

capacity of transmission mains and permits a more uniform and economical operation of the water supply, treatment, and pumping facilities. It is calculated as the maximum volume of water used during a single hour, multiplied by 24 hours.

- **Minimum Day (MinD):** Minimum day usage is becoming increasingly significant relative to issues of water quality in the distribution system. It is the basis for evaluating the maximum water age in the distribution system, which coincides with greatest degradation of water quality. It is calculated at the minimum volume of water used during a single day.

5.2. Historical Water Demands

Historical water production and water billing data was used in combination with population and employment to develop an understanding of historical water use in the Service Area.

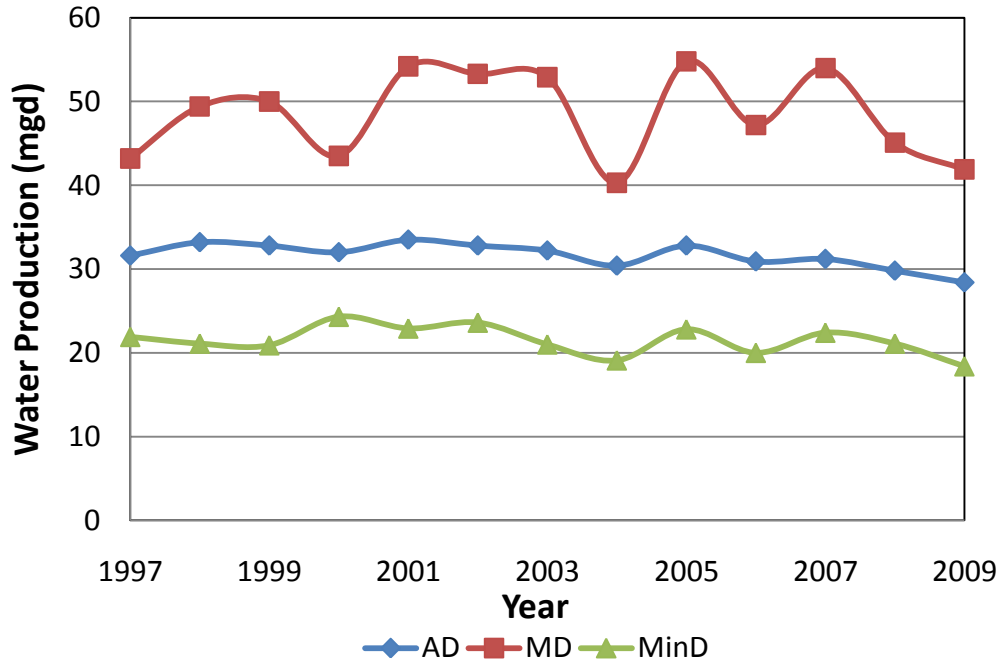
5.2.1 Historical Production

Table 5 summarizes historical water production by MWU with the characteristics provided for AD, MD, and MinD, which are shown graphically on Figure 2. Although population has increased by approximately 9 percent since 1997, the AD demand does not show a similar increasing trend.

Table 5 - Historical Water Production

Year	AD (mgd)	MD (mgd)	MD:AD Ratio	MinD (mgd)	MinD:AD Ratio
1997	31.6	43.2	1.37	21.9	0.69
1998	33.2	49.4	1.49	21.1	0.64
1999	32.8	50.0	1.52	20.9	0.64
2000	32.0	43.5	1.36	24.3	0.76
2001	33.5	54.2	1.62	22.9	0.68
2002	32.8	53.3	1.62	23.6	0.72
2003	32.2	52.9	1.64	21.0	0.65
2004	30.4	40.3	1.33	19.1	0.63
2005	32.8	54.8	1.67	22.8	0.69
2006	30.9	47.2	1.53	20.0	0.65
2007	31.2	54.0	1.73	22.4	0.72
2008	29.8	45.1	1.51	21.1	0.71
2009	28.4	41.9	1.48	18.4	0.65
Average	31.7	48.4	1.53	21.5	0.68
Maximum	33.5	54.8	1.73	24.3	0.76
Minimum	28.4	40.3	1.33	18.4	0.63
mgd – million gallons per day					

Figure 2 - Historical Water Production



The decreasing trend in AD demands in the past few years, despite the growth in population, cannot fully be attributed to conservation efforts as summer climate characteristics are also a factor in water usage. Table 6 and Figure 3 summarize the SD water production, average temperature, and precipitation data for years 1999 through 2009. As can be seen from this information, water demand can vary as a function of temperature or rainfall. For example, 2005 represents a relatively hot and dry year with higher water demand rates. A series of figures detailing the 7-day averages of production, temperature, precipitation are provided in *Attachment C*.

Table 6 - Summer Production vs. Climate Data

Year	Summer ¹ Demand (mgd)	Average Summer ¹ Temperature (°F)	Summer ¹ Rainfall (inches)
1999	38.2	67	9.6
2000	35.8	67	9.1
2001	39.4	67	16.1
2002	40.0	69	7.8
2003	39.3	68	9.1
2004	33.5	66	11.0
2005	39.8	70	7.1
2006	35.7	67	13.0
2007	37.4	69	20.3
2008	35.1	68	9.3
2009	32.2	67	8.6
Average	36.9	68	11.0
Maximum	40.0	70	20.3
Minimum	32.2	66	7.1

¹ Data from July - September

Figure 3 - Water Production and Climate Influence

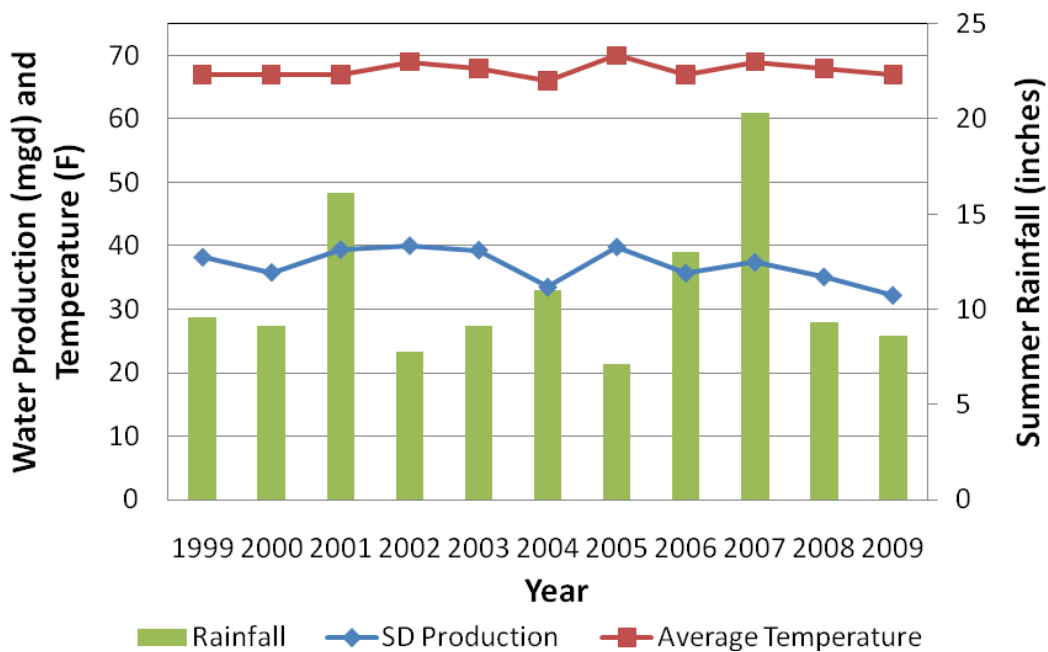


Figure 3 seems to indicate that there is no obvious direct correlation between cumulative summer rainfall (labeled on the secondary y-axis) and summer demand. Other summer

precipitation factors may impact summer water demand such as time of rainfall or other unidentified spring climatic conditions. It appears that there may be a correlation between summer demand and average summer temperature. In any case, historical water demand records indicate a significant variation in summer demands. Making long term water demand projections based on short term trends may not be appropriate.

5.2.2 Non-Revenue Water

In addition to billed water use, there is an unaccounted-for (or non-revenue) water component. Non-revenue water is defined as the difference between total water production and metered sales. The American Water Works Association (AWWA) Manual 36 – Water Audits and Loss Control Programs (Third Edition 2009) identifies two types of non-revenue water, real losses and apparent losses. Real losses include incidents where the water is never put to a beneficial use, such as pipeline leaks and tank overflow spills. Apparent losses include losses related to meter inaccuracies and non-metered water use, such as system flushing, fire fighting, or unauthorized connections.

As shown in Table 7, estimated non-revenue water in the MWU system varies between 6.9 and 12.8 percent. This is within industry standards for a well operated water distribution system. An average non-revenue water of 10 percent will be assumed for future water production projections. The implementation of AMI and monthly billing will provide a much higher level of detail regarding water use patterns. When this information is available, the MWU may want to complete a water audit to identify the sources and possible options for reduction of non-revenue water.

Table 7 - Historical Non-Revenue Water

Year	Non-Revenue Water (mgd)	Non-Revenue Water (percent)
1997	3.1	9.7
1998	3.3	9.9
1999	2.4	7.2
2000	2.9	8.9
2001	3.9	11.7
2002	3.7	11.1
2003	3.5	10.8
2004	3.4	11.1
2005	4.2	12.8
2006	2.2	6.9
2007	3.4	10.7
2008	3.1	10.4
2009	2.2	7.6
2010	2.5	10.0
Average	3.1	9.9
Maximum	4.2	12.8
Minimum	2.2	6.9

5.2.3 Demand by User Class

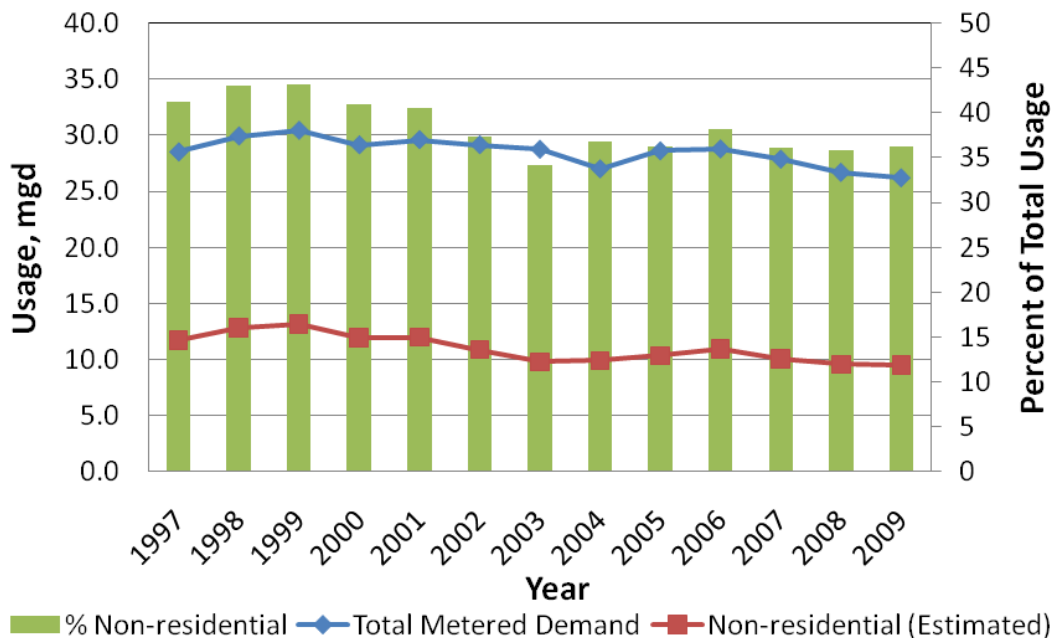
Year end reports provided to the Public Service Commission (PSC) detail metered water sales data for residential and non-residential (commercial, wholesale, industrial, and other use categories). Residential include single family and duplex housing. Multi-family use is included as commercial sales in these reports. Based on MWU information on multi-family accounts, it was assumed that approximately 75 percent of the water recorded as commercial (but not other non-residential use) was related to multi-family use. Table 8 summarizes the historical water use characteristics for residential and non-residential categories. As can be seen from this table, the average ratio of non-residential water use to the total metered use is approximately 38 percent. Figure 4 graphically depicts the ratio of non-residential water use to the total use.

Table 8 - Historical Metered Water - Residential vs. Non-Residential

Year ¹	Average Day (mgd)			
	All Residential		Non Residential	Total
	Residential Meters (Single Family & Duplexes)	Commercial Meters (Apartments) ²		
1997	8.8 (31%)	7.9 (28%)	11.8 (41%)	28.5
1998	9.0 (30%)	8.1 (27%)	12.9 (43%)	29.9
1999	9.1 (30%)	8.2 (27%)	13.1 (43%)	30.4
2000	9.1 (31%)	8.2 (28%)	11.9 (41%)	29.2
2001	9.3 (31%)	8.3 (28%)	12.0 (41%)	29.6
2002	9.6 (33%)	8.7 (30%)	10.9 (37%)	29.1
2003	10.0 (35%)	9.0 (31%)	9.8 (34%)	28.7
2004	9.0 (33%)	8.1 (30%)	9.9 (37%)	27.1
2005	9.9 (35%)	8.3 (29%)	10.4 (36%)	28.6
2006	9.4 (33%)	8.4 (29%)	11.0 (38%)	28.8
2007	9.4 (34%)	8.4 (30%)	10.1 (36%)	27.9
2008	9.0 (34%)	8.1 (30%)	9.6 (36%)	26.7
2009	8.8 (33%)	8.0 (30%)	9.5 (36%)	26.2
2010	8.4 (33%)	8.0 (32%)	8.9 (35%)	25.3
Average	9.2 (33%)	8.3 (29%)	10.8 (38%)	28.3
Maximum	10.0	9.0	13.1	30.4
Minimum	8.4	7.9	8.9	25.3

¹ Year 1997 -2004 data from 2006 Master Plan, Table 3-4.
² Estimated as approximately 75 percent of metered commercial sales

Figure 4 - Historical Non-residential Usage



5.2.4 Large Users

Large water users, non-residential customers that use more than 100,000 gpd on an annual average basis, make up nearly 75 percent of the total non-residential demand. These include the University of Wisconsin, Oscar Mayer Foods, hospitals, government entities, and wholesale customers. Table 9 summarizes large user demands over the last 10 years. Generally a large water user’s demand is fairly consistent. The University of Wisconsin – Madison has been actively pursuing water conservation for the last few years and has seen a significant decrease in demand since 2002. In addition to water demands for the University buildings, there is also a Cogeneration Facility on campus that typically uses cooling water from Lake Mendota, but could put a demand as large as 2 mgd on the system during a drought when lake levels are inadequate to supply cooling water. The wholesale customers: Shorewood Hills and Maple Bluff see a more significant change from year to year, which is likely because their service area is primarily residential customers with variable seasonal demands.

In the distribution system computer model, large user demands will be point loaded so that the model properly handles these usually large demands. It is assumed that large user demand will be reduced in the future consistent with other non-residential water conservation efforts.

Table 9 - Existing Largest Water Customers

Water Customer	AD Demand (mgd)										
	2000 ¹	2001 ¹	2002 ¹	2003 ¹	2004 ¹	2005 ¹	2006	2007	2008 ²	2009 ²	Average
University of Wisconsin ³	4.18	4.15	4.02	3.74	3.72	3.00	3.25	2.78	2.96	2.78	3.46
Oscar Mayer Foods	1.30	1.54	1.10	1.00	0.85	1.59	1.6	1.34	1.45	1.34	1.31
Government (Federal, State, County) ³	--	--	--	--	--	--	0.81	0.78	0.72	0.67	0.75
City of Madison ³	--	--	--	--	--	--	0.64	0.70	0.72	0.61	0.67
Covance	0.11	0.12	0.12	0.18	0.17	0.17	0.16	0.15	0.21	0.19	0.16
Meriter/Madison General Hospital	0.16	0.14	0.13	0.15	0.14	0.15	0.12	0.17	0.15	0.19	0.15
St. Mary's Hospital	0.13	0.14	0.17	0.17	0.11	0.12	0.14	0.15	0.14	0.15	0.14
Webcrafters, Inc.	0.16	0.15	0.11	0.21	0.16	0.16	0.13	0.15	0.13	0.13	0.15
Airgas Merchant Gases ⁴	0.12	0.12	0.13	0.16	0.13	0.13	0.12	0.11	0.11	0.11	0.12
V.A. Hospital	--	--	--	--	--	--	--	0.08	0.10	0.09	0.08
Aramark	0.17	0.16	0.11	0.10	0.10	0.10	0.10	0.10	0.09	0.09	0.11
Forest Products Lab	--	--	--	--	0.11	0.11	0.09	0.09	0.09	0.09	0.10
Superior Health Linens	--	--	--	--	--	--	--	0.08	0.09	0.08	0.08
Danisco, USA	--	--	--	--	--	--	--	0.09	0.09	0.09	0.09
American Family Insurance	--	--	--	--	--	--	0.09	0.08	0.08	0.08	0.08
Shorewood Hills ⁵	0.25	0.22	0.38	0.17	0.16	0.21	0.21	0.16	0.19	0.16	0.19
Maple Bluff ⁶	0.16	0.20	0.18	0.18	0.20	0.18	0.31	0.24	0.17	0.23	0.19
Waunona Sanitary District No. 2	0.14	0.14	0.12	0.10	0.14	0.14	0.13	0.09	0.12	0.14	0.13

Notes: -- Unavailable or unreported.
¹ 2006 Master Plan, Table 36
² Email from MWU October 14, 2010
³ Multiple facilities.
⁴ Previously AGA Gas
⁵ Village of Shorewood Hills experienced a large system leak in 2002. Average does not include 2002 data.
⁶ Village of Maple Bluff had very high usage in 2006. Average does not include 2006 data.

5.2.5 Historical Unit Demands

In order to determine the appropriate residential unit demand and determine the sensitivity of the assumptions related to the multi-family usage, two methodologies were used to compare residential unit demand results.

The first methodology was based on the assumptions from the 2006 Master Plan. The residential usage (single family and duplexes) was used in conjunction with the number of residential metered accounts and an assumed housing density to determine a residential per-capita unit water use. This data is shown in Table 10 and is only representative of single-family and duplex residential use. The housing density used in this table beginning at 2.43 persons per household for year 1997 with a decreasing trend is based on the City 2005 Comprehensive Plan, Volume I where the data shows that in 2000 the single-family household density is 2.4 persons per household and has been declining at a rate of .01 persons per year since 1980. The limitation of this methodology is that it only provides information on the residential unit usage for single-family and duplex use and it is also highly dependent upon the household density assumptions.

Table 10 - Unit Residential Water Use Calculations (Method 1)

Year	Average Residential Metered Use (mgd)	Number of Residential Metered Accounts	Single Family Housing Density ¹	Calculated Residential Metered Population	Per-Capita Residential Water Use (gpcd)
1997	8.8	46,944	2.43	114,100	77.3
1998	9.0	47,513	2.42	115,000	78.0
1999	9.1	48,143	2.41	116,000	78.3
2000	9.1	49,029	2.4	117,700	77.3
2001	9.3	50,033	2.39	119,600	77.5
2002	9.7	51,250	2.38	122,000	78.8
2003	10.0	52,391	2.37	124,200	80.3
2004	9.0	53,454	2.36	126,200	71.6
2005	9.9	53,454	2.35	125,600	78.9
2006	9.4	55,270	2.34	129,300	72.7
2007	9.4	55,730	2.33	129,900	72.2
2008	9.0	56,033	2.32	130,000	69.2
2009	8.8	56,244	2.31	129,900	67.3
2010	8.4	56,448	2.30	129,830	64.70
Average	9.2	--	--	--	71.7
Maximum	10.0	--	--	--	80.3
Minimum	8.4	--	--	--	64.7

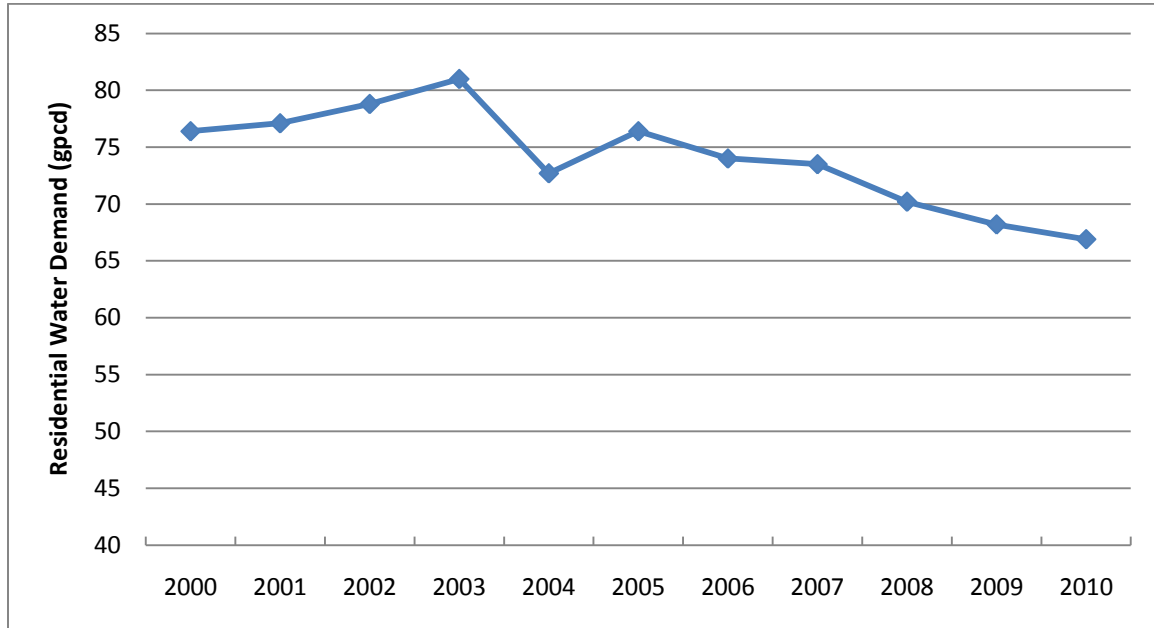
¹ Housing Density is declining as described in the Madison Comprehensive Plan

The second methodology used to estimate residential unit water demand uses the service area population estimates provided in Table 3 and the estimated total residential usage provided in Table 8. The resulting data is provided in Table 11. Both methods produce similar results, but since Method 2 is able to incorporate all residential use and is less dependent on household density assumptions it was used for estimating historical water use. As shown in Figure 5, there is currently a downward trend in per capita residential water use. This is primarily the result of water saving fixtures and increased conservation awareness. Results for 2003 and 2005, however, show that there can still be years with higher demands. In order to ensure adequate capacity for a higher demand year, a unit demand of 74 gpcd was chosen for future demands without conservation (High). This is consistent with the highest demand year out of the last 5 years of available data.

Table 11 - Unit Residential Water Use Calculations (Method 2)

Year	Residential Usage (mgd)	Service Population	Per-Capita Residential Water Use (gpcd)
2000	17.3	226,149	76.4
2001	17.6	228,463	77.1
2002	18.3	231,739	78.8
2003	18.9	233,721	81.0
2004	17.2	235,945	72.7
2005	18.3	238,929	76.4
2006	17.8	240,437	74.0
2007	17.8	241,982	73.5
2008	17.1	243,760	70.2
2009	16.7	244,804	68.2
2010	16.4	245,208	66.9
Average	17.6	--	74.1
Maximum	18.9	--	81.0
Minimum	16.4	--	66.9

Figure 5- Historical Residential Water Demand



5.2.6 System Peaking Factors

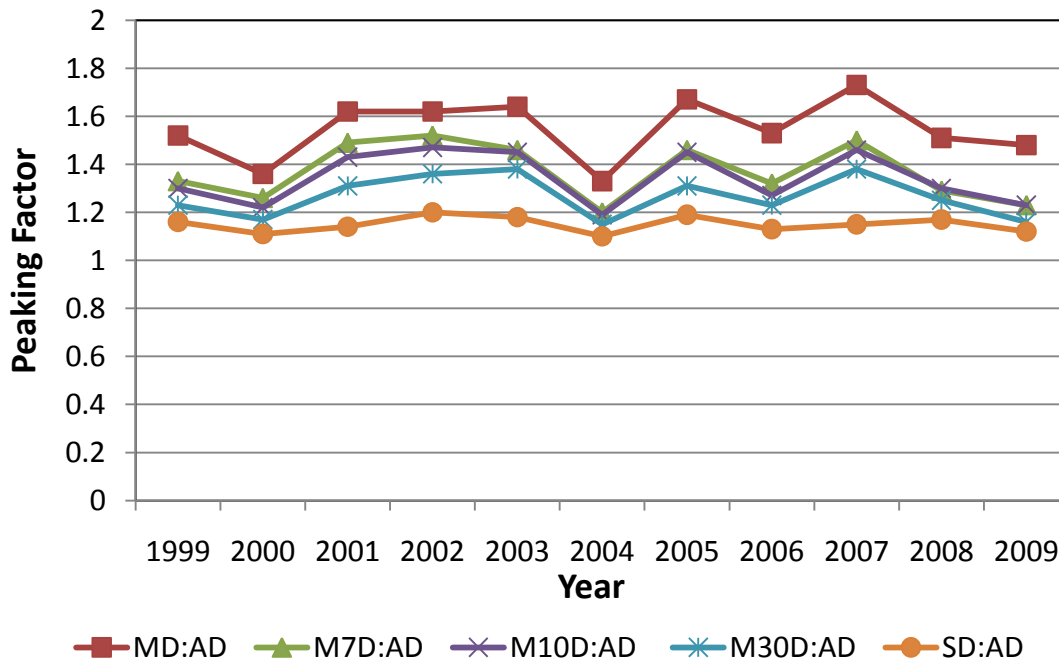
In order to determine typical system peaking factors, pumping data from years 1999 to 2009 was evaluated. Peaking factors for each year of data were calculated and summarized in Table 12 and are shown in Figure 6 for the following conditions:

- MD Demand vs. AD Demand
- M7D Demand vs. AD Demand
- M10D Demand vs. AD Demand
- M30D Demand vs. AD Demand
- SD Demand vs. AD Demand

Table 12 - Historical Peaking Characteristics from Well Pumping Data

Year	Peaking Period				
	MD:AD	M7D:AD	M10D:AD	M30D:AD	SD:AD
1999	1.52	1.33	1.30	1.23	1.16
2000	1.36	1.26	1.22	1.17	1.11
2001	1.62	1.49	1.43	1.31	1.14
2002	1.62	1.52	1.47	1.36	1.20
2003	1.64	1.46	1.45	1.38	1.18
2004	1.33	1.20	1.19	1.15	1.10
2005	1.67	1.46	1.45	1.31	1.19
2006	1.53	1.32	1.27	1.23	1.13
2007	1.73	1.50	1.46	1.38	1.15
2008	1.51	1.29	1.30	1.25	1.17
2009	1.48	1.23	1.23	1.16	1.12
Average	1.53	1.37	1.34	1.27	1.15
Maximum	1.73	1.52	1.47	1.38	1.20
Minimum	1.33	1.20	1.19	1.15	1.10

Figure 6 - Historical Summer Peaking Factors



The 11 years of peaking factor data shows that there is no trending and that the MD:AD factors show considerable variability. This data supports the system overall MD:AD design peaking factor of 1.74 used in the 2006 Master Plan.

5.2.7 Existing Demands by Service Zone

The spatial distribution of usage by service zone was evaluated and compared to the characteristics presented in the 2006 Master Plan. Consumption and demand distribution characteristics by service zone were evaluated for the AD, M7D, and MD where SCADA data allowed for the evaluation of the spatial distribution in demand.

Table 13 presents the distribution in metered consumption for the AD condition based upon the metered sales data. As indicated in this table, the percent of total AD consumption by service zone in 2003 is similar to the percent of total AD consumption by service zone in 2008. Notable exceptions are service zones with growth and/or boundary modifications (Zone 1, Zone 3, Zone 10, and Zone 11).

Table 13 - Consumption by Service Zone (AD)

Service Zone	2003 AD Consumption ¹ (mgd)	Percent of System Consumption (%)	2008 AD Consumption ² (mgd)	Percent of System Consumption (%)
1	0.29	0.9	0.37	1.4
2 ³	0.12	0.4		
3 ³	0.19	0.6	0.88	3.4
4	1.24	3.8	1.08	4.1
5	0.05	0.2	0.03	0.1
6W ⁴	22.54	69.6	10.13	38.7
6E ⁴			6.84	26.1
7	4.19	12.9	3.19	12.2
8	2.52	7.8	2.21	8.5
9	0.66	2.0	0.59	2.3
10	0.49	1.5	0.67	2.6
11	0.08	0.3	0.18	0.7
Total	32.37	100.0	26.18	100.0

¹ From Metered Sales Allocated for the 2006 Master Plan
² From 2008 Metered Sales Allocation but using current Service Zone Boundaries
³ Zone 2 and Zone 3 were merged in 2010 (called Zone 3) with some of Zone 6E being incorporated
⁴ Zone 6 was essential split by the closure of isthmus valves into Zone 6E and Zone 6W

Table 14 presents the spatial distribution of demand by service zone for a M7D demand condition for 2003 and a week of high demand in August of 2010. At the time of this study not all information had been evaluated for 2010 but it is assumed that this week

presents a condition comparable to a M7D demand condition. This table is based upon the compilation of SCADA data with estimates of booster pump station flows. As can be seen from this table the spatial distribution of demand during the M7D is similar in 2010 to the week in 2003 except that there has been a slight increase in Zone 9. The spatial distribution of demand is also slightly greater in Zone 3 but this is likely due to incorporation of some of Zone 6E into Zone 3.

Table 14 - Demand by Service Zone (M7D)

Service Zone	Year 2003 M7D Demand (mgd) ¹	Percent of System Demand (%)	Year 2010 Week of Aug 1 Demand (mgd) ²	Percent of System Demand (%)
1	0.6	1.4	0.41	1.2
2 ³	0.33	0.8	1.31	3.8
3 ³	0.48	1.1		
4	1.77	4.3	1.48	4.3
5	0.07	0.2	0.04	0.1
6W ⁴	26.8	64.5	10.11	29.6
6E ⁴			10.23	29.3
7	3.38	8.1	3.67	10.6
8	5.66	13.6	4.34	12.6
9	0.91	2.2	1.73	5.0
10	1.16	2.8	0.91	2.6
11	0.42	1.0	0.28	0.8
Total	41.58	100	34.52	100.0

¹ Year 2003 from 2006 Master Plan based upon previous evaluation of SCADA information
² Year 2010 estimated from SCADA information for week of August 1
³ Zone 2 and Zone 3 were merged in 2010 (called Zone 3) with some of Zone 6E being incorporated
⁴ Zone 6 was essentially split by the closure of 3 of 4 isthmus valves in 2006 into Zone 6 E and Zone 6 W

Table 15 presents the spatial distribution of demand by service zone for a MD demand condition in 2003 and a 24-hour period of high demand between August 3rd and August 4th of 2010. The calculated demands are based upon the compilation of SCADA data with estimates for booster pump station flows. This table shows that the spatial distribution of demand during the MD in 2003 is similar to the day in 2010. The spatial distribution of demand is slightly greater in Zone 3 but this is due to incorporation of some of Zone 6E into Zone 3. Note that some data for Booster Pump Station 106 did not appear to be captured correctly in the SCADA system in 2010 and therefore estimates were made regarding the run times of this Booster Pump Station using best judgment based on typical operations. Since these estimates effect the spatial distribution calculations for Zone 7 and Zone 6 W, underestimates could result in the lowered calculated percent of total demand for Zone 7 in 2010.

Table 15 - Demand by Service Zone (MD)

Service Zone	Year 2003 MD Demand (mgd) ¹	Percent of System Demand (%)	Year 2010 August 3-4 Daily Demand (mgd) ²	Percent of System Demand (%)
1	0.76	1.4	.44	1.2
2 ³	0.42	0.8	1.28	3.5
3 ³	0.61	1.2		
4	2.25	4.3	1.55	4.2
5	0.09	0.2	0.03	0.1
6W ⁴	34.09	64.5	13.38	36.0
6E ⁴			10.66	28.7
7 ⁵	4.3	8.1	2.27	6.1
8	7.19	13.6	4.65	12.5
9	1.15	2.2	1.72	4.6
10	1.48	2.8	0.93	2.5
11	0.53	1.0	0.29	0.8
Total	52.87	100.00	37.17	100.00

¹ Year 2003 from 2006 Master Plan based upon previous evaluation of SCADA information
² Year 2010 estimated from SCADA information for August 3-4.
³ Zone 2 and Zone 3 were merged in 2010 (called Zone 3) with some of Zone 6E being incorporated
⁴ Zone 6 was divided by the closure of isthmus valves into Zone 6E and Zone 6W
⁵ Missing data from Booster Pump Station 106 required some assumptions on run times for this station. Consequently demands may be underestimated for Zone 7 and overestimated for Zone 6W.

Tables 13 through 15 support the assumptions used in the 2006 Master Plan regarding the spatial distribution of demand by service zone and indicate that using the design values indicated in the 2006 Master Plan will be appropriate to project the future spatial distribution of demand and the peaking factors in conjunction with TAZ population and employment data.

5.3. Future Water Demands

A range of water demand projections were developed using the criteria and information provided in previous sections of this memorandum, information from the 2006 Master Plan, and the TAZ population and employment projections provided by the planning department.

Since MWU recently implemented a conservation plan, its ultimate effectiveness is unknown. Therefore, a range of water demand rates was considered including: High – no conservation, Medium – half of goal, and Low – full conservation. Based on the goals in the MWU Conservation Plan, it was assumed that conservation (based on the activities outlined in the MWU Conservation Plan) will be achieved by 2020. Additional calculation details are included in Attachment D.

5.3.1 Residential Unit Demands

The residential unit demands that were used to determine projected water demands are based on the information provided in Tables 11 and 12 and the recommended conservation unit demand. The residential unit demands to be used are listed below:

- High: All design years (2010, 2015, 2030, and Buildout) – 74 gpcd
- Mean: (halfway between the high and low residential unit demands)
 - 2010 – 74 gpcd
 - 2015 – 71 gpcd
 - 2030 and Buildout – 67 gpcd
- Low:
 - 2010 – 72 gpcd – This value is based on the average of the last five years of data and assumes that conservation has had a slight impact in residential unit demands in this average.
 - 2015 – 65 gpcd. This value is based on the assumption that half of the conservation will be achieved by 2015 (i.e. halfway between no 72 gpcd at 2010 , and conservation goals achieved fully in 2020 or 58 gpcd. This breaks down to $72 + 58$ divided by 2, or 65 gpcd)
 - 2030 and Buildout – 58 gpcd (achieved by 2020)

5.3.2 Non-residential Demands

To determine the non-residential demand component, an initial non-residential to total metered demand percentage of 38 percent was used for year 2010 (high – 11.2 mgd, medium – 11.2 mgd, and low – 10.9 mgd). This is the average value of the last 10 years, as shown in Table 8. Based on the implementation of conservation practices, it was assumed that between now and 2020 this demand volume would stay constant. (demand from increased employment would be off-set by increased conservation).

To project the non-residential water use for years 2030 and Buildout, the 2020 gallons-per-employee-per-day (gped) unit usage was calculated using the 2020 non-residential water use and the 2020 total employment from the TAZ data. The 2020 non-residential unit usage rate was calculated as 46.8 gped for the high and mean (11.2 mgd divided by 238,000 employees) and 45.7 for the low value (10.9 mgd divided by 238,000 employees).

5.3.3 System-Wide Average Day Demands

Using the residential unit demands provided in Section 5.3.1, the non-residential demand assumptions described in the previous section, and a non-revenue component of 10 percent, the range of water demands was calculated and is provided in Table 16 and is shown graphically in Figure 6. Although the low, mean, and high total demand for 2010 in Table 16 are higher than recent metered water use shown in Table 8, this is due to the addition of non-revenue demand and the need for water demand projections to reflect likely demands in a warmer and drier than average year.

Table 16 – System-Wide Average Day Water Demand Projections (mgd)

Design Year	Range Category	Residential Demand	Non-Residential Demand	Non-Revenue Demand	Total Demand
Year 2010	High	18.2	11.2	3.3	32.7
	Mean	18.2	11.2	3.3	32.7
	Low	17.7	10.9	3.2	31.8
Year 2015	High	19.0	11.2	3.4	33.5
	Mean	18.2	11.2	3.3	32.7
	Low	16.7	10.9	3.1	30.7
Year 2030	High	21.3	12.3	3.7	37.3
	Mean	19.3	12.3	3.5	35.1
	Low	16.7	12.0	3.2	31.9
Buildout	High	28.2	15.1	4.8	48.1
	Mean	25.5	15.1	4.5	45.1
	Low	22.1	14.7	4.1	41.0

Figure 7 – Average Day Projected Demands

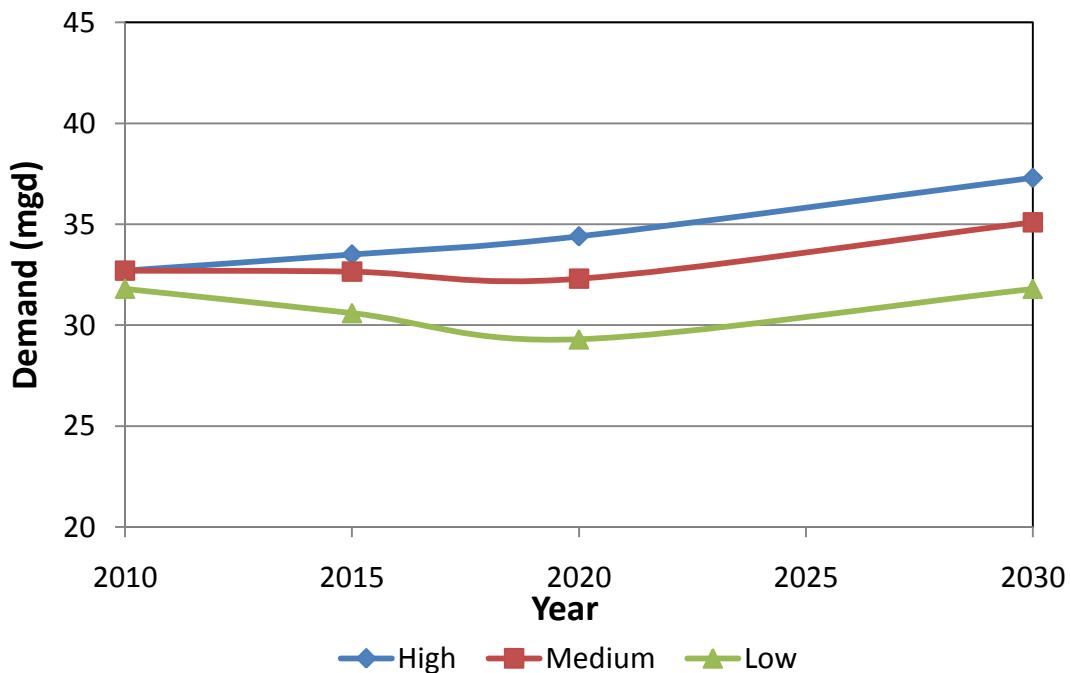


Figure 6 provides an illustration of the AD demand projections following the criteria outlined in the previous sections. This figure illustrates that for the low demand projections (conservation goals are fully achieved) the AD water use in 2030 will be approximately the same as current water use. Conservation gains will compensate for the additional projected demand resulting from residential and non-residential growth.

Although MWU and the public have embraced the Conservation Plan and have already begun to see some results, the ability to reach and maintain the low demand assumptions has not been shown. The demand range for mean and low will be used as the basis for the future projections and recommendations. MWU should monitor progress on conservation goals and adjust demand assumptions if needed on future distribution system evaluations.

5.3.4 System Design Peaking Factors

The system design peaking factors for MD, and MH are presented in the 2006 Master Plan and have not changed for this study as they have been supported by the evaluations presented in this memorandum. The M10D peaking factor, which was not used in the 2006 Master Plan, was determined for this project as the 90th percentile value (10-year return interval) using the last eleven years of data shown in Attachment E. The M10D peaking factors were added to the evaluation for use during water age hydraulic analyses. Table 17 lists the system-wide design peaking factors based on existing water usage and Table 18 summarizes the system-wide demand projections.

Table 17 - System Peaking Factors

Condition	Peaking Factor
M10D ¹	1.47
MD ²	1.74
MH ²	2.15

¹ 90th percentile value from past 11 years of data
² From the 2006 Master Plan

Table 18 - System Wide Water Demand Projection Range (mgd)

Design Year	AD ¹	M10D ²	MD ²	MH ²
Year 2010	31.8 – 32.7	46.4 – 47.7	55.3 – 56.8	68.3 – 70.2
Year 2015	30.6 – 32.7	44.7 – 47.7	53.3 – 56.8	66.8 – 70.2
Year 2030	31.8 – 35.1	46.5 – 51.3	55.4 – 61.1	68.5 – 75.4
Buildout	40.9 – 45.1	59.8 – 66.0	71.1 – 78.6	87.9 – 97.1

¹ From Table 16 (Mean and Low Range Category)
² Peaking Factor from Table 17.

6. EAST SERVICE AREA DEMAND AND SUPPLY ANALYSIS

Although understanding the system-wide demand and peaking factors is an important step in establishing overall demands, the East Service Area is the focus of this project. The demands and peaking factors for each service zone (zone) were determined so that the facility needs for each zone can be properly evaluated.

The East Service Area includes all MWU facilities east of the Yahara River shown in Attachment A. The East Service Area includes zones: 1, 3, 4, 5, and 6E. Since MWU has plans to combine zones 1 and 3 and call it Zone 3, these areas will be referred to as Zone 3 for this memo. Because of the need to maintain a mass balance in the hydraulic model for the entire system both the East and West Service Area demands and peaking factors are included in the discussion and projections in this section.

6.1. East Service Area Average Day Demand

In order to divide the projected demand up by zone, GIS was used to combine population and employment by TAZ with the zone boundaries. The demands for large users were assigned to the proper TAZ so that they were also included in the demand projections. Table 19 summarizes the demands by zone.

Table 19 – Average Day Demand Projections by Zone (mgd)

Zone	2010	2015	2030	Build Out
East Service Area				
3	2.2	2.3 - 2.4	3.2 – 3.5	7.5 – 8.3
4	1.3	1.2 - 1.3	1.4 – 1.5	4.0 – 4.4
5	0.1	0.1	0.1	0.1
6E	7.0 – 7.2	6.6 – 7.1	6.5 – 7.2	7.2 – 8.0
East Service Area Total	10.6 – 10.8	10.2 – 10.9	11.2 – 12.3	18.7 – 20.8
West Service Area				
6W	12.4 - 12.7	11.8 - 12.5	11.0 – 12.0	11.2 – 12.3
7	3.8 – 4.0	3.6 - 3.9	3.4 – 3.9	3.4 – 3.8
8	3.3 – 3.4	3.2 – 3.4	3.5 – 3.8	3.9 – 4.2
9	0.7	0.7	0.8 – 0.9	0.6 – 0.7
10	0.9	1.0 – 1.1	1.7 – 1.8	2.6 – 2.9
11	0.1	0.2	0.3	0.4 – 0.5
West Service Area Total	21.2 – 21.8	20.4 - 21.8	20.7 – 22.7	22.2 – 24.4
System-Wide Total	31.8 – 32.6	30.6 – 32.7	31.9 – 35.0	40.9 – 45.2

6.2. Peaking Factor by Service Zone

Peaking factors vary by zone across the water distribution system as a function of the size and the mix of residential and non-residential customers. In general, as the amount of demand within a zone increases its peaking factors will decrease. This section describes the methodology used to project peaking factors for each zone. Table 20 summarizes the selected peaking factors.

Developing peaking factors for each zone requires reviewing both the existing peaking factors for each zone and balancing the rate of growth between zones. In addition, in order to satisfy the mass balance equations in the hydraulic model, the weighted average of the peaking factors must match the system-wide peaking factors selected in Section 5.3.4

Table 20 – Peaking Factor by Zone

Zone	M10D	MD	MH
East Service Area			
3	1.57	1.87	2.31
4	1.51	1.80	2.22
5	3.23	3.85	4.76
6E	1.34	1.60	1.98
West Service Area			
6W	1.28	1.52	1.88
7	1.67	1.99	2.46
8	1.65	1.96	2.42
9	2.02	2.40	2.97
10	1.83	2.18	2.69
11	2.52	3.00	3.71
System-Wide	1.47	1.74	2.15

6.2.1 Maximum Day Peaking Factors

During the 2006 Master Plan two curves were developed that predict the MD to AD peaking factors for a zone based on the demand within the zone (2006 Master Plan, Figure 3-10). Because of water use characteristics, zones 6 and 4 use a curve with lower peaking factors than the remainder of the zones. These curves were used as the first step to predict the peaking factor for each zone. For each zone, the average AD demand for 2010, 2015, and 2030 was used and the corresponding MD/AD peaking factor read. Finally, the peaking factors for all zones were adjusted to meet the MD demand.

6.2.2 Maximum 10 Day and Maximum Hour Peaking factors.

Once the MD/AD peaking factors were determined the M10D and MH peaking factors were calculated by maintaining the same ratios of M10D/MD and MH/MD that can be derived from the peaking factors in Table 17:

- M10D to MD: 0.84
- MH to MD: 1.24

6.3. Design Demand Condition

Table 21 summarizes the projected demand conditions for each service area. Note that the sum of the individual zones will not be exactly equal to the demand projected for the system-wide as a result of the methodology used to select the peaking factors by zone and the different rates of growth in the various zones. Checks made confirm that the sum of the zones is within 2 percent (most are within 1 percent) of the system-wide projected demand.

Table 21 – Projected Demand Summary by Service Area

Zone	2010			2015			2030		
	M10D	MD	MH	M10D	MD	MH	M10D	MD	MH
Low Demand									
East Side									
3	3.4	4.1	5.0	3.6	4.2	5.2	5.0	6.0	7.4
4	1.9	2.3	2.8	1.9	2.2	2.7	2.1	2.5	3.1
5	0.2	0.3	0.3	0.2	0.3	0.3	0.2	0.2	0.3
6E	9.5	11.3	13.9	8.9	10.6	13.1	8.8	10.5	12.9
East Side Total	15.0	18.0	22.0	14.6	17.3	21.3	16.1	19.2	23.7
West Side									
6W	15.8	18.8	23.3	15.1	17.9	22.1	14.1	16.7	20.7
7	6.4	7.6	9.5	6.0	7.2	8.9	5.8	6.9	8.5
8	5.4	6.4	7.9	5.3	6.3	7.8	5.8	6.9	8.5
9	1.4	1.7	2.1	1.4	1.6	2.0	1.6	1.9	2.3
10	1.7	2.0	2.4	1.8	2.2	2.7	3.0	3.6	4.5
11	0.3	0.4	0.50	0.4	0.5	0.6	0.6	0.7	0.9
West Side Total	31.0	36.8	45.7	29.9	35.6	44.1	30.9	36.7	45.4
System-Wide Total	46.0	54.8	67.7	44.5	52.9	65.4	47.0	55.9	69.1
Mean Demand									
East Side									
3	3.5	4.2	5.2	3.8	4.5	5.6	5.6	6.6	8.2
4	2.0	2.3	2.9	2.0	2.4	2.9	2.3	2.7	3.3
5	0.2	0.3	0.3	0.2	0.3	0.4	0.2	0.3	0.3
6E	9.7	11.5	14.3	9.5	11.3	14.0	9.7	11.5	14.3
East Side Total	15.4	18.3	22.7	15.5	18.5	22.9	17.8	21.1	26.1
West Side									
6W	16.3	19.4	23.9	16.0	19.0	23.5	15.3	18.3	22.6
7	6.6	7.9	9.7	6.5	7.7	9.6	6.5	7.7	9.5
8	5.6	6.6	8.2	5.6	6.7	8.2	6.3	7.5	9.3
9	1.4	1.7	2.1	1.5	1.8	2.2	1.8	2.1	2.6
10	1.7	2.0	2.5	2.0	2.3	2.9	3.4	4.0	5.0
11	0.3	0.4	0.5	0.4	0.5	0.6	0.7	0.9	1.1
West Side Total	31.9	38.0	46.9	32.0	38.0	47.0	34.0	40.5	50.1
System-Wide Total	47.3	56.3	69.6	47.5	56.5	69.9	51.8	61.6	76.2

6.4. Well Capacity Analysis

The final evaluation of well supply and water distribution system capacity will be done in conjunction with the hydraulic modeling when the impact of system interaction and operation on the ability to meet demands can be looked at more holistically. This evaluation compares only the well capacity with the demands. It does not consider operational or vulnerability issues associated with the unit wells, storage, booster pumping or pressure reducing valves. In addition, this evaluation assumes the unit wells can pump at the design rate for an extended period of time.

The Level of Service Memo written for the East Side Project identified two criteria for evaluating well capacity in the East Service Area:

- AD demand is \leq 50 percent of well capacity
- MD demand is \leq well capacity with one well out of service (2 wells for Zone 6E)

The more stringent MD requirements for Zone 6E, with respect to well outages is based on: MWUs operational experience, there are currently 7 operating wells in Zone 6E, and Zone 6E wells provide service or backup service to Zones 3, 4, and 5. Experience indicates that a single well out of service in Zone 6E due to planned maintenance or mechanical breakdown occurs annually. For engineering planning purposes to ensure system reliability, considering a second well outage in Zone 6E due to a power outage or other natural disaster is reasonable and prudent.

Table 22 summarizes the target capacities for the various zones based on their connections to other zones and projected demands.

Table 22 – East Side Required Well Capacity (mgd)

Zone	2010		2015		2030	
	AD ¹	MD ²	AD ¹	MD ²	AD ¹	MD ²
3	4.4	4.1 – 4.2	4.6 - 4.8	4.2 – 4.5	6.4 – 7.1	6.0 – 6.6
4	2.6	2.3	2.4 - 2.6	2.2 – 2.4	2.8 – 3.0	2.5 – 2.7
5	--	--	--	--	--	--
6E ³	14.2 – 14.6	11.6 – 11.9	13.4 - 14.3	10.9 – 11.6	13.1 – 14.4	10.7 – 11.8
East Side Total	21.2 – 21.6	19.0 – 19.4⁴	20.4 - 21.7	18.3 – 19.5⁴	22.3 – 24.5	20.2 – 22.1⁴

¹ Two times AD demand

² MD demand

³ Totals include Zone 5 demand

⁴ Based on recommendations in the 2006 Water Master Plan this total includes 1.0 mgd that is transferred to Zone 6W from Zone 6E on MD

Table 23 summarizes the available well capacity available to meet demands by zone. Note that the well capacity is based on current production and could change in the future as a result of changes in aquifer water quality or production capacity.

Table 23 – East Service Area 2010 Well Capacity

Zone	Unit Well	Capacity (mgd)	Capacity with largest well out of service
3	25	3.0	0.0 ³
4	9	2.5	0.0 ³
5	--	--	--
6E	7 ¹	3.0	11.3 ⁴
	8 ¹	2.4	
	11	3.0	
	13	3.0	
	15	3.0	
	23	1.4	
	29	1.5 ²	
	6E Total	17.3	
East Service Area Total		22.8	16.8⁵ – 18.4⁶ 13.8⁶ – 15.4⁷
¹ Treatment needs to be added to wells 7 and 8 to meet water quality goals. ² There are plans to expand the production capacity of this well to 3.1 mgd ³ Supply would be provided from Zone 6E ⁴ Two wells out of service ⁵ Assumes that one well is out in 6E and Well 25 is out of service ⁶ Assumes that two wells are out in 6E and Well 25 is out of service ⁷ Takes into account increasing capacity of Well 29 to 3.1 mgd			

A brief evaluation of the ability of the existing unit wells to meet the level of service criteria for each zone follows.

6.4.1 Service Zone 3 Well Capacity Evaluation

Well 25 is currently the only well serving Zone 3. A booster pump station near Well 29 provides limited ability to transfer water from Zone 6E. Based on the projections in Table 22 and the established supply criteria, Zone 3 relies on Zone 6E for 1.4 mgd supply on the average day. In the event that Well 25 is taken out of service, Zone 3 will be entirely dependent on Zone 6E. The Zone 3 demands will be included in the Zone 6E supply capacity analysis. The 2006 Water Master Plan provides for a new pump station to transfer water from Zone 6E to Zone 3 so this is projected to be a long term supply arrangement. Several additional wells are included in the 2006 Master Plan to meet future demands in Zone 3 and reduce reliance on Zone 6E. Currently for planning purposes, 1.4 mgd will be assumed to be provided to Zone 3 from Zone 6E.

6.4.2 Service Zone 4 Well Capacity Evaluation

Zone 4 is currently served by a single well, Well 9. Current water demand projections for Zone 4 indicate that well capacity falls slightly short of meeting the AD water supply criteria. If Well 9 is out of service, at the present time Zone 4 will be completely reliant on Zone 6E. MWU is in the process of adding a second well to Zone 4. It is anticipated that the new well will be in production by 2013 and have a capacity of 3.1 mgd. This additional well will reduce the Zone 4 reliance on Zone 6E and provide capacity to support growth in Zone 4. A pump station that would move water from Zone 4 to Zone 6E is proposed in the 2006 Water Master Plan. Following construction of the new well in Zone 4, Zone 4 will have adequate capacity to meet AD and MD demands through 2030.

Zone 5 does not have any unit wells and is served entirely from Zone 6E. Zone 5 serves a small area with limited growth potential. The projected water demands in Zone 5 are small and can be easily met from Zone 6E facilities.

6.4.3 Service Zone 6E Well Capacity Evaluation

The existing water demand in Zone 6E is approximately double the sum of the other 3 east side zones combined. Additionally Zone 6E wells provide water supply support zones 3 and 5 and some water also flows from Zone 6E to Zone 6W across the Yahara River to meet peak demands. To adequately assess the water supply capacity on the east side, all of these water transfers must be considered in the planning process.

The projected average day capacity requirement for the East Side varies from 21.2 to 21.6 for 2010 demands and 22.4 – 24.5 for 2030 demands. Firm supply capacity from existing wells, depending on assumptions and criteria and the increase in capacity at Well 29, ranges from 13.8 mgd to 18.4 mgd. This results in an estimated short fall range from 2.8 to 10.7 mgd for Zone 6E. To meet minimum estimated water supply requirements for reliability and redundancy considering that the conservation program is successful, a minimum of one well is required in Zone 6E for existing demands and additional wells may be required in the future if Well 29 cannot be expanded or conservation goals are not met.

This well supply evaluation only takes into account the basic supply capacity of the east side wells. It does not consider the hydraulic capacity of the distribution system and its ability to effectively move water around the system. It is anticipated that system capacity and ability to move water around to where it is needed will dictate the number and location of any additional wells in Zone 6E that will be required to meet 2030 water supply demands. Computer modeling will determine siting and pumping capacity required for proposed facilities.

7. SUMMARY AND CONCLUSIONS

This memorandum presents water demand projections for a range of conditions. These water demand projections were developed based on the following data sources:

- Population and employment projections and spatial distributions provided by the Madison Area Transportation and Planning Board
- Population estimates from the State of Wisconsin – Department of Administration Demographic Service Center
- Daily well pump data from 1999 through 2009
- 2006 Water Master Plan
- MWU Conservation Plan
- Historical usage, Production, and Meter data provided to the Public Service Commission of Wisconsin (PSCW) and presented in the Year End Reports.

The water demand projections provided in this memorandum are intended to represent a range of demands that the utility can expect to experience. Based on MWU and the community's commitment to conservation the mean and low projections were chosen for future demands. The MWU should monitor progress on conservation goals and adjust demand assumptions if needed on future distribution system evaluations.

Because the items identified in the Conservation Plan are primarily aimed at reducing indoor water usage, they will primarily impact AD demands. Peak water demands associated with summer irrigation may not change significantly resulting in higher peaking factors. Although existing peaking factors were used for this evaluation, MWU should continue to monitor peaking factors to identify any trends in demand patterns.

To provide a preliminary evaluation of the well capacity for the East Side, the existing well capacity was compared to the projected demands through 2030 and the level of service criteria. This evaluation only compared the well capacity with the demands. It did not consider operational or vulnerability issues associated with the unit wells, storage, booster pumping or pressure reducing valves. The well capacity evaluation highlighted the importance of Zone 6E facilities to increase the reliability of zones 3, 4, and 5 and identified an existing minimum shortfall of about 3.0 mgd in Zone 6E.

Based on the information gathered and developed during this study, it is recommended that MWU:

1. Monitor demands and peaking factors, as they implement the Conservation Plan
2. Monitor the impact of weather patterns on outdoor water use
3. Plan for a replacement well for Well 3 (Well 45) to replace lost system capacity
4. Investigate the feasibility of increasing the filtration capacity of Well 29
5. In the event that the capacity at Well 29 cannot be increased, consider adding a well in Zone 3 in the near term that can supplement supply to Zone 6E

More information on supply requirements regarding well location will be developed during the distribution system computer model evaluation

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ATTACHMENT A

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ATTACHMENT B

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ATTACHMENT C

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Attachment D

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Attachment E

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